

# ASSESSING EXPECTED ACCURACY OF PROBE VEHICLE TRAVEL TIME REPORTS

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(Reviewed by the Urban Transportation Division)

**ABSTRACT:** The use of probe vehicles to provide estimates of link travel times has been suggested as a means of obtaining travel times within signalized networks for use in advanced travel information systems. Past research in the literature has provided contradictory conclusions regarding the expected accuracy of these probe-based estimates, and consequently has estimated different levels of market penetration of probe vehicles required to sustain accurate data within an advanced traveler information system. This paper examines the effect of sampling bias on the accuracy of the probe estimates. An analytical expression is derived on the basis of queuing theory to prove that bias in arrival time distributions and/or in the proportion of probes associated with each link departure turning movement will lead to a systematic bias in the sample estimate of the mean delay. Subsequently, the potential for and impact of sampling bias on a signalized link is examined by simulating an arterial corridor. The analytical derivation and the simulation analysis show that the reliability of probe-based average link travel times is highly affected by sampling bias. Furthermore, this analysis shows that the contradictory conclusions of previous research are directly related to the presence or absence of sample bias.

## INTRODUCTION

The successful wide scale deployment of advanced traveler information systems (ATIS) and advanced traffic management systems depends on the ability to obtain and subsequently disseminate information that accurately reflects network traffic conditions. Many different techniques for assessing traffic conditions have been proposed. However, one method in particular, namely, the use of vehicles that are capable of transmitting link travel times to the traffic management center, has received considerable attention. The use of probe vehicles enables a sample of the travel times experienced by all vehicles traversing the link to be obtained. This paper examines the use of probe vehicles on signalized links and addresses the critical question of "How accurately do the probe vehicle travel times (sample) reflect the travel times of all the vehicles (population) that traversed the link?"

A number of researchers have previously investigated the expected reliability of probe travel time reports. Van Aerde et al. (1993) developed an analytical expression for the reliability of probe travel times for signalized links and verified these expressions using simulated data. These expressions, which assume that probe reports represent an independent random sample from the traffic stream, indicate that as the number of probe reports in a period increases, the sample mean approaches the population mean.

The same assumption was used by other researchers in determining the required level of market penetration or number of probe vehicles (Boyce et al. 1991a,b; Turner and Holder 1995; Srinivasan and Jovanis 1996).

Sen et al. (1997a,b) examined field data collected from probe vehicles as part of the ADVANCE project. On the basis of a statistical analysis of probe link travel times, they found that probe reports are not independent, and therefore, regardless of the sample size, the sample mean does not approach

the population mean. They concluded that only "a small number of probe reports within a 5-min interval yields a standard error that is not substantially improved by making the number of probes much larger."

The conclusions reached by Van Aerde et al. (1993) and Sen et al. (1997) appear to be contradictory. This paper will show that both results are indeed correct, but each is appropriate only for specific traffic and sampling conditions, and neither result can be held as a generalization for all traffic network conditions.

The remainder of this paper consists of three components. In the next section it is shown from fundamental queuing theory that bias in the probe sample leads to a sample mean that does not asymptotically approach the population mean, regardless of the sample size. Following this, simulation data are used to illustrate the impact of sample bias on a signalized arterial. Finally, conclusions are made regarding the importance of these findings for the design of probe-based ATIS and advanced traffic management systems.

## ANALYTICAL ESTIMATION OF EXPECTED DELAY OF PROBE VEHICLES

This section provides a theoretical estimate of the mean travel time experienced by a probe vehicle traversing a signalized arterial link. The objective is to show that the mean travel time of the probe vehicles (the subpopulation from which samples are taken for estimation) may be different from the mean travel time of all the vehicles (population). The travel time that a vehicle experiences when traversing a signalized link consists of two components, namely, the running time and the delay caused by the signal control. In the following theoretical derivation, we will assume that the mean running times of the probe vehicles and the general vehicles are the same and we will focus on the difference in mean delay between probes and all vehicles.

## Assumptions and Notations

The delay that a probe vehicle experiences when it travels through a signalized approach depends on a number of factors, including the arrival flow rate and distribution, signal timings, and the time when the vehicle arrives at the approach. In a real application environment, many of these factors are random variables. As a result, the travel time reported by probe

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vehicles would likely be subject to large variation. For the purpose of illustrating the effect of sample bias, we will consider an idealized intersection approach consisting of a single through lane controlled by a signal with known timings. The approach has unlimited space for queuing and has a constant saturation flow rate. Furthermore, it is assumed that vehicle arrivals at the approach are uniformly distributed and consist only of passenger car units (pcu). The other notations are described as follows:

- Signal Timing Parameters

- $c_y$  = cycle time (s)
- $g$  = effective green interval (s)
- $r$  = effective red interval (s)
- $\lambda = g/c_y$

- Arrival Flow

- $q_g$  = average arrival flow rate during effective green interval (pcu/s)
- $q_r$  = average arrival flow rate during effective red interval (pcu/s)
- $q$  = average arrival flow rate during cycle time (pcu/s). Defined as

$$q = \frac{q_g \cdot g + q_r \cdot r}{c_y} \quad (1)$$

- Capacity

- $s$  = saturation flow rate (pcu/s)
- $c_a$  = capacity (pcu/s), determined by  $s\lambda$
- $x$  = degree of saturation during the cycle time, defined as  $q/c_a$

- Probe Vehicle Flow

- $P_g$  = proportion of probe vehicles among all vehicles arriving during effective green interval. The probe arrival rate during effective green interval is therefore  $q_g \cdot P_g$  (probe pcu/s)
- $P_r$  = proportion of probe vehicles among all vehicles arriving during effective red interval. The probe arrival rate during the red interval is therefore  $q_r \cdot P_r$  (probe pcu/s)
- $q_p$  = average probe arrival flow rate during cycle time (pcu/s), defined as

$$q_p = \frac{P_g \cdot q_g \cdot g + P_r \cdot q_r \cdot r}{c_y} \quad (2)$$

- $\phi$  = ratio of the proportion of probe vehicle arrivals during the effective green interval to the proportion of probe vehicle arrivals during the effective red interval, defined as  $\phi = P_g/P_r$

### Distribution of Delay

Fig. 1 illustrates the arrival rate for all vehicles and for probe vehicles only. Consider the case that a probe vehicle is randomly sampled from all probe vehicles arriving at the approach during the cycle time. The arrival time of the sampled probe vehicle, noted as  $T$ , would be a stepwise uniformly distributed random variable, and its distribution can be described by (3)

$$f_T(t) = \begin{cases} \frac{P_r \cdot q_r}{q_p \cdot c_y} & \text{if } 0 < t \leq r \\ \frac{P_g \cdot q_g}{q_p \cdot c_y} & \text{if } r < t \leq c_y \end{cases} \quad (3)$$

It should be noted that if both  $P_r$  and  $P_g$  are replaced by 1.0 in (3), the result is the probability density function (PDF) of

the arrival time of a general vehicle sampled from all arriving vehicles (population).

For a vehicle arriving at the approach at a given time  $t$ , its delay (noted as  $d$  for general vehicles and  $d_p$  for probe vehicles) can be determined based on deterministic queuing theory (as shown in Fig. 2).

$$d_p = d = \begin{cases} r + \left(\frac{q_r}{s} - 1\right) \cdot t & \text{if } 0 < t \leq r \\ \frac{q_r \cdot r}{s} \left(\frac{t_c - t}{t_c - r}\right) & \text{if } r < t \leq t_c \\ 0 & \text{if } t_c < t \leq c_y \end{cases} \quad (4)$$

where  $t_c$  = time when the queue is cleared and can be determined by

$$t_c = r \left(1 + \frac{q_r}{s - q_g}\right) \quad (5)$$

In the case that a vehicle is randomly selected from the arriving flow, its delay would also be a random variable with its distribution depending on the distribution of its arrival time [(3)] (Fig. 3). Denote  $D_p$  as the delay of a randomly selected probe vehicle, and  $D$  as the delay of a randomly selected general vehicle. The following section discusses the derivation of the distribution of  $D_p$ . Note that the distribution of  $D$  can be easily obtained from the distribution of  $D_p$  by setting  $P_r = P_g = 1.0$ .

First, the sampled vehicle may experience no delay and the probability of this outcome is equal to the probability that the vehicle arrives during the time interval  $[t_c, c_y]$ , which can be determined from (3)

$$P(D_p = 0) = P(t_c < T < c_y) = (c_y - t_c) \cdot \frac{P_g \cdot q_g}{q_p \cdot c_y} = \frac{P_g \cdot q_g}{q_p \cdot c_y} \left(g_e - \frac{q_r \cdot r}{s \cdot q_g}\right) \quad (6)$$

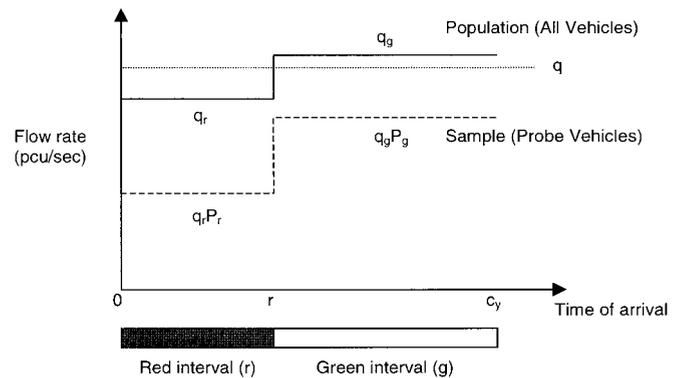


FIG. 1. Illustration of Arrival Rates for Probe Vehicles and All Vehicles during Red and Green Intervals

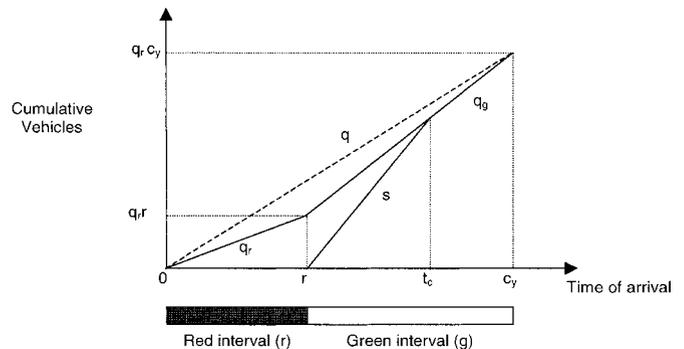


FIG. 2. Deterministic Queuing Diagram

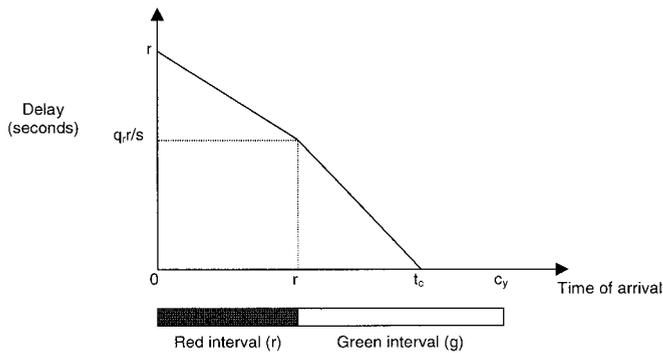


FIG. 3. Uniform Queue Delay as Function of Vehicle Arrival Time

Second, as shown in (4), the vehicle may experience a delay that decreases linearly from  $q_r r/s$  to zero when the arrival time increases from  $r$  to  $t_c$ . The probability that the vehicle would experience a delay greater than zero and less than or equal to  $q_r r/s$  is therefore equal to the probability that the vehicle arrives during the time interval  $[r, t_c]$  [(7)]

$$P\left(0 < D_p \leq \frac{q_r \cdot r}{s}\right) = P(r < T \leq t_c) = (t_c - r) \cdot \frac{P_g \cdot q_g}{q_p \cdot c_y}$$

$$= \frac{q_r \cdot r}{(s - q_g)} \cdot \frac{P_g \cdot q_g}{q_p \cdot c_y} \quad (7)$$

Because the arrival time is uniformly distributed over the arrival time interval  $[r, t_c]$ , delay should also be uniformly distributed with its PDF as shown in (8)

$$f_{D_p}(d_p) = \frac{P\left(0 < D_p < \frac{q_r \cdot r}{s}\right)}{\left(\frac{q_r \cdot r}{s} - 0\right)} = \frac{P_g \cdot q_g \cdot s}{q_p \cdot c_y \cdot (s - q_g)}, \quad \left(0 < d_p \leq \frac{q_r \cdot r}{s}\right) \quad (8)$$

Similarly, the probability that the vehicle would experience a delay greater than  $q_r r/s$  and less than or equal to  $r$  can be determined based on (4)

$$P\left(\frac{q_r \cdot r}{s} < D_p < r\right) = P(0 < T < r) = (r - 0) \cdot \frac{P_r \cdot q_r}{q_p \cdot c_y} = \frac{P_r \cdot q_r \cdot r}{q_p \cdot c_y} \quad (9)$$

The PDF of the delay within this regime is

$$f_{D_p}(d_p) = \frac{P\left(\frac{q_r \cdot r}{s} < d_p < r\right)}{\left(r - \frac{q_r \cdot r}{s}\right)} = \frac{P_r \cdot q_r \cdot s}{q_p \cdot c_y \cdot (s - q_r)}, \quad \left(\frac{q_r \cdot r}{s} < d_p \leq r\right) \quad (10)$$

In summary, the delay of sampled probe vehicle is a mixed discrete and continuous random variable with its PMF represented by (6), (7), and (9), and the PDF represented by (8) and (10).

### Mean Delay of Probe Vehicles

With the given distribution functions, the mean delay of probe vehicles  $E[D_p]$  can be obtained through the following mathematical expectations:

$$E[D_p] = 0 \cdot P(D_p = 0) + \int_0^{(q_r/s)r} f_{D_p}(x) x dx + \int_{(q_r/s)r}^r f_{D_p}(x) x dx \quad (11)$$

Based on (8) and (10), (11) can be rewritten as

$$E[D_p] = \frac{s \cdot r^2}{2 \cdot q_p \cdot c_y} \left[ \frac{P_r \cdot q_r}{s - q_r} \left(1 - \frac{q_r^2}{s^2}\right) + \frac{P_g \cdot q_g}{s - q_g} \left(\frac{q_r^2}{s^2}\right) \right] \quad (12)$$

Consider a more idealized situation where no platoon progression exists and the arrival flow rate during the effective green interval is the same as the arrival rate during the effective red interval (i.e.,  $q_g = q_r = q$ ). Then (12) can be simplified to (13)

$$E[D_p] = \frac{r^2}{2 \cdot c_y} \cdot \frac{1 + x^2 \lambda^2 (\phi - 1)}{\left(1 - \frac{q}{s}\right) [1 + \lambda (\phi - 1)]}$$

$$= E[D] \cdot \frac{1 + x^2 \lambda^2 (\phi - 1)}{1 + \lambda (\phi - 1)}, \quad (x \leq 1.0) \quad (13)$$

From (13), it can be observed that if the probe arrival ratio  $\phi$  is equal to 1 (i.e., a randomly selected probe can be considered as a general vehicle), the resulting equation is the well-known expression for uniform delay for all vehicles (Hurdle 1984; Teply et al. 1995). This implies that the expected delay of the probes is equal to the expected delay of all vehicles (population).

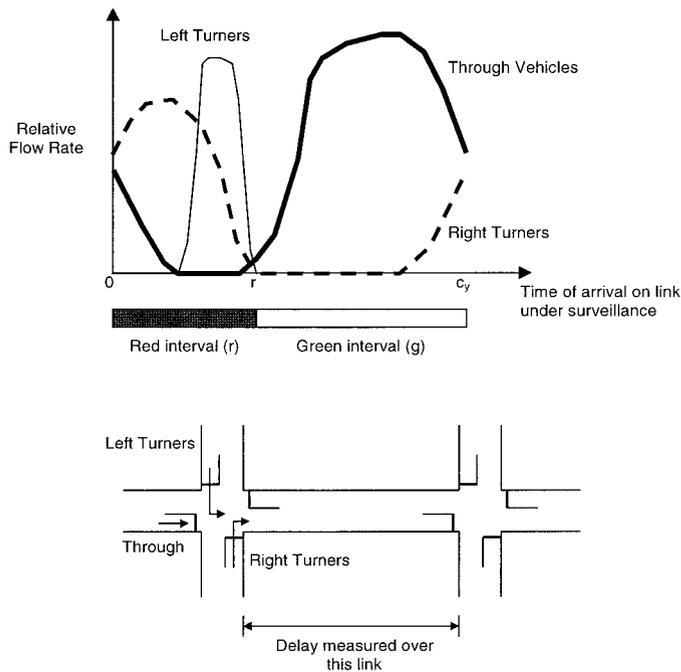
However, when the proportion of probe arrivals during the green interval  $P_g$  is not equal to the proportion of probe vehicle arrivals during the red interval  $P_r$ , then  $\phi \neq 1$  and  $E[D_p]$  is no longer equal to the expected mean of the population  $E[D]$ . Physically, this means that if a disproportionate number of probe reports are received from probes that arrive during the green or red interval, then the sample is no longer random, but is biased. Consequently, the average delay computed from the probe reports is also biased and will asymptotically approach the probe mean  $E[D_p]$ , but not the population mean  $E[D]$ , even if the number of probe reports is very large.

The theoretical analysis in this section has examined an idealized signalized intersection in which the delay associated with only a single link departure movement has been considered. However, the results are equally applicable to intersections with more than one outbound movement (e.g., left-turn, through, and right-turn movements), if each outbound movement is considered individually. The next section examines the potential sources of bias that are likely to be encountered in field conditions, and addresses the issue of multiple outbound movements.

### SOURCES OF SAMPLE BIAS

An important practical issue then is to determine under what conditions the probe sample can be considered a biased sample. The previous section has shown, based on a theoretical derivation, that for an idealized intersection arrival time bias in the sample will lead to a systematic bias in the estimate of the population mean. If we consider that the objective is to estimate the population mean link travel time, where the population consists of all vehicles traversing the link regardless of the movement used to exit the link, then for a typical signalized link, the sample of probe reports can become biased in two ways:

1. First, the distribution of link entry times for probes may differ from the population. The time at which a vehicle enters the upstream end of a link depends largely on the turning movement required to access the link and the traffic controls impacting that movement. For example, consider the network illustrated in Fig. 4 in which the intersection bounding the upstream end of the link is signalized and consists of four approaches. Vehicles accessing the link for which the delay is being measured do so via one of three possible movements, namely, a



**FIG. 4. Arrival Time Bias as Function of Link Entry Movement**

left turn from the cross street; a right turn from the cross street; or a through movement from the main street. Each of these movements is controlled by the upstream traffic signal and by gap acceptance behavior for opposed movements (i.e., left turn on green, and right turn on red). As illustrated in Fig. 4, the distribution of link entry times is different for each of the three movements. Consequently, if the proportion of probe vehicles varies with each movement, then it also follows that the probe link entry time distribution will be different from that associated with the population. Because delay at the downstream intersection is a function of arrival time, a bias in arrival time will also result in a bias in delay.

2. The second cause of bias is associated with the movement used to exit the link (i.e., left turn; through, or right turn) at the downstream intersection. Typically, each

movement experiences a different average delay. If the proportion of probe vehicles varies with each exit movement, then the sample is biased as a result of over- or undersampling an exit movement that experiences a delay that is greater than or less than the population average.

Consequently, for a link bounded by a four-leg intersection at the upstream and downstream ends, there exist nine distinct subpopulations (one associated with each combination of entry and exit movement), each with its own set of travel time characteristics. If the proportion of probes varies across these nine subpopulations, then a bias will result.

### EFFECT OF SAMPLE BIAS WITHIN SAMPLE NETWORK

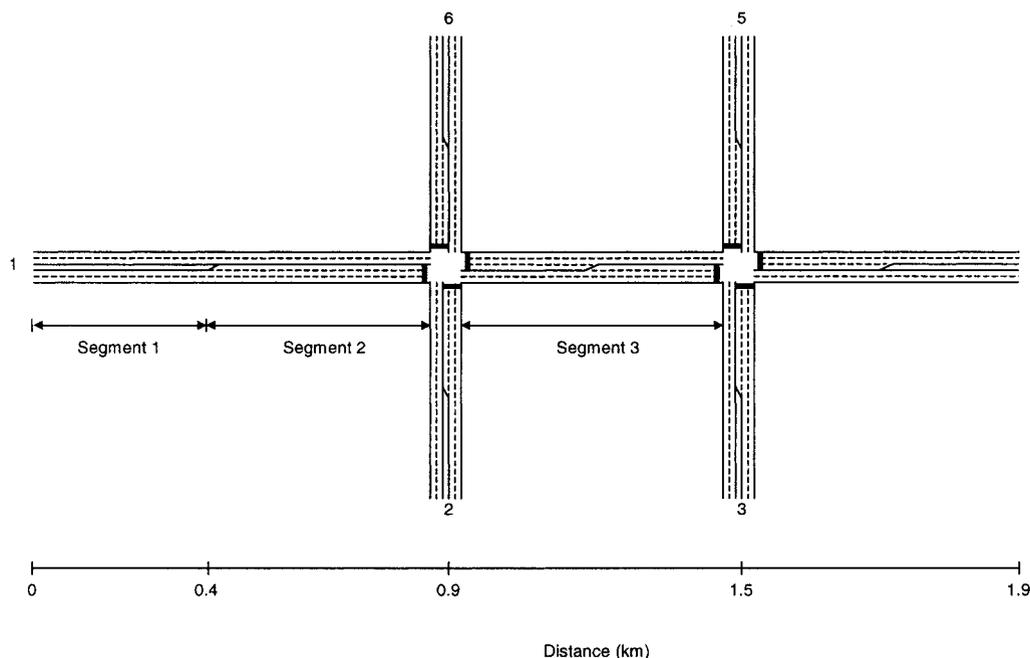
To illustrate the potential magnitude of the sample bias, a simple linear network was modeled using the INTEGRATION traffic simulation model (Van Aerde et al. 1996). The network, illustrated in Fig. 5, consists of a single arterial roadway that is intersected by two cross streets. Each intersection is controlled by a fixed-time signal ( $c_y = 120$  s,  $g = 75$  s for the arterial,  $g = 37$  s for the cross street, and offset = 0 s).

The network is modeled for 1 h with constant (time invariant) demands. Vehicles are generated at all origin zones with negative exponentially distributed headways. The O-D traffic demands between each of the six zones are provided in Table 1. The application of these demands to the network results in the intersection approaches experiencing V/C ratios ranging from approximately 0.3 to 0.75. Consequently, all of the intersection approaches operate in an undersaturated mode.

The time to traverse each link segment, the unique vehicle identification number, the time when the vehicle departed the link (i.e., time of probe report), and the vehicle's origin and destination were recorded for each vehicle. This log represented the travel times experienced by the entire vehicle population.

Three probe sampling scenarios were defined as follows:

1. Unbiased: All O-D pairs were sampled at the same level of market penetration.



**FIG. 5. Arterial Test Network Configuration**

**TABLE 1. O-D Traffic Demands for Test Network<sup>a</sup>**

Origin zone (1)	Destination Zone						Total (8)
	1 (2)	2 (3)	3 (4)	4 (5)	5 (6)	6 (7)	
1	—	300	162	808	81	100	1,450
2	75	—	31	154	15	600	875
3	38	5	—	100	200	7	350
4	920	120	150	—	200	160	1,550
5	192	25	500	200	—	33	950
6	250	400	8	38	4	—	700
Total	1,475	850	850	1,300	500	900	5,875

<sup>a</sup>Vehicles per hour.

- Biased 1–6: Only vehicles traveling between Origin 1 and Destination 6 were sampled.
- Biased 2–4: Only vehicles traveling between Origin 2 and Destination 4 were sampled.

For each sampling scenario, simulations were conducted for 22 levels of market penetration (i.e., 0.0–0.3 in 0.02 increments and 0.4–1.0 in 0.1 increments). For each test, probe reports were aggregated into 5-min periods (12 periods in total). The mean segment travel time computed on the basis of the probe reports, and the number of probe reports within each period were recorded for each test. The results are examined for vehicles traveling eastbound on link Segments 1, 2, and 3 only (Fig. 5).

Segment 1 represents a link that is not affected by traffic signals at either its upstream or downstream, and for which only a single entry movement and exit movement is possible. Therefore, a biased sample (based on over- or undersampling a subpopulation) is not possible. Furthermore, because the link is not controlled by a signal, the link travel times are not expected to exhibit a great amount of variation.

Segment 2 represents a link for which bias can only arise as a result of over- or undersampling the downstream exit movements.

Segment 3, having a signalized intersection at both the upstream and downstream boundaries, is susceptible to bias as a result of over- or undersampling any of the nine possible combinations of entry and exit movements.

Fig. 6 illustrates the mean travel time for Segment 1 estimated from probe reports received during each 5-min period, as a function of the number of probe reports. The 95% confidence limits (C.L.) of the estimated mean segment travel time are also illustrated. These C.L. are computed from (14) using

the entire vehicle population under the assumption that probe reports represent a randomly selected sample from a single infinite population. For Segment 1, there is no opportunity for bias by over- or undersampling a specific entry or exit movement, and so this assumption is valid

$$C.L. \text{ of } \bar{x} = \bar{x} \pm z \left( \frac{\text{var}(x_i)}{n} \right) \quad (14)$$

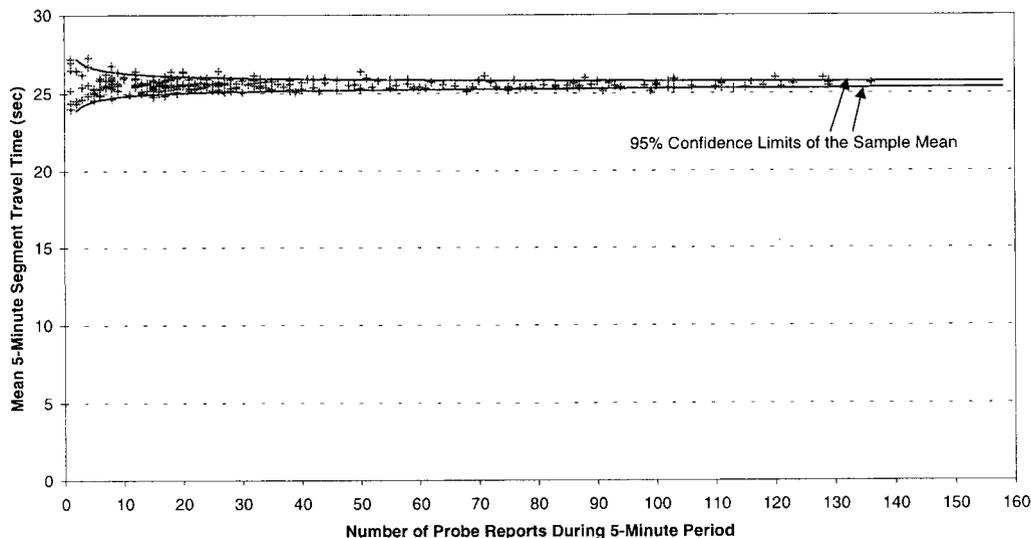
where  $\bar{x}$  = mean segment travel time computed from the entire population of vehicles traversing the segment during the 1-h simulation;  $\text{var}(x_i)$  = variance of the individual vehicle segment travel times about  $\bar{x}$ ;  $n$  = number of probe reports received during the time interval; and  $z$  = normal standard deviate associated with the confidence limits (i.e.,  $z = 1.96$  for a confidence limit of 95%).

Three observations can be made from Fig. 6. First, the mean travel times are all within a very small range of between 23 and 28 s. This is expected because no signal impacts this link. Second, the 5-min average travel times are distributed between the confidence limits with no apparent bias. Third, the confidence limits of the sample mean show an initial rapid reduction in the error of the estimate as the number of probe reports increases. Furthermore, the error tends to zero as the number of reports approaches infinity.

Fig. 7 illustrates the mean 5-min travel times obtained from probe reports for biased and unbiased samples. From these results it is evident that having a biased sample results in mean travel time estimates that do not represent the travel time experience of the population. In particular, when sampling from only those vehicles traveling from Origin 1 to Destination 6, the resulting sample mean travel times are much larger than the population mean travel times. This is expected as this sample of vehicles is required to make a left turn at the downstream end of Segment 2, and consequently, experience a much greater delay than the population of vehicles traversing Segment 2.

Fig. 8 illustrates the effect of bias in the arrival time distribution between probe vehicles and general vehicles traversing Segment 3. Mean travel times from two samples are presented. The unbiased sample reflects the experience of the true population. The biased sample only reflects vehicles that are traveling between Origin 2 and Destination 4. From the results in Fig. 8 it is evident that the biased sample travel times consistently underestimate the population travel time.

While the biases presented in this example network can be considered to be extreme, they do serve to illustrate the po-



**FIG. 6. Mean 5-min Travel Times and 95% Confidence Limits for Unbiased Sample (Segment 1)**

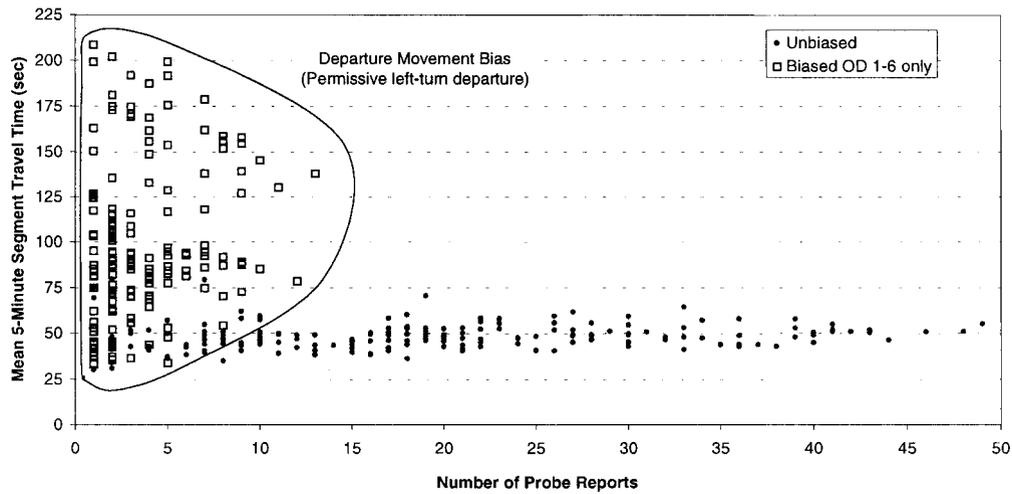


FIG. 7. Mean 5-min Travel Times from Unbiased and Biased Samples as Function of Number of Probe Reports (Segment 2)

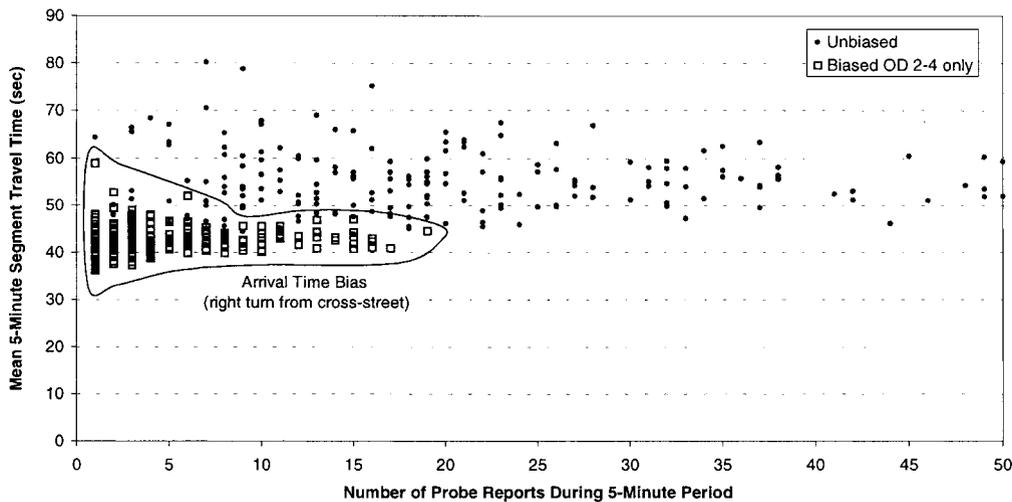


FIG. 8. Mean 5-min Travel Times from Unbiased and Biased Samples as Function of Number of Probe Reports (Segment 3)

tential extent of the problem. Furthermore, the results from Sen et al. (1997b), discussed earlier in this paper, are based on field data in which all probe vehicles traversed a set of links by using the same link entry and link exit movements. Thus, the sample used in their analysis has a level of bias that is similar to that associated with the examples provided in this paper. The findings described within this paper demonstrate that the conclusions of Sen et al. that “a small number of probe reports within a 5-minute interval yields a standard error that is not substantially improved by making the number of probes much larger,” should not be generalized.

## CONCLUSIONS AND RECOMMENDATIONS

Previous research in the literature has provided seemingly contradictory conclusions regarding the accuracy of mean link travel times estimated from probe vehicle reports. The apparent disagreement between these results gives rise to confusion among practitioners and may lead to inappropriate ATIS design decisions.

This paper has examined the issue of the accuracy of mean travel times as estimated from probe vehicles. More specifically, this paper has shown that under conditions when the probe vehicles represent a biased sample, the sample mean does not approach the population mean. It has also been shown that for a typical link, which is bounded at both the upstream and downstream ends by a signalized intersection, the population of vehicles can be divided into nine subpopulations, each associated with a unique turning movement combination

for entering and exiting the link. If the proportion of probe vehicles varies between these nine subpopulations, the probe reports represent a biased sample and error associated with the sample mean may remain quite large, even when the sample size is large. However, if the proportion of probe vehicles is nearly constant across all nine subpopulations, then the sample is unbiased, and the probe reports can be considered to represent independent samples from the population. In this case, standard sampling theory holds, and the error of the sample mean decreases as the sample size increases.

Thus, the identification of sample bias enables the apparently inconsistent results of previous research reported in the literature to be more clearly interpreted. Specifically, the work of Sen et al. (1997b) can be seen as applicable to scenarios in which the probe reports constitute a biased sample of the population. Conversely, the work of Van Aerde et al. (1993) is applicable when the probe reports are an unbiased sample of the population.

It has been shown that the degree to which the probe reports represent a biased sample is critical in assessing the reliability of the sample mean as an estimate of the population mean. Therefore, it is recommended that further research focus on developing methods that can be applied under field conditions to quantify the degree of bias associated with a sample of probe reports. Furthermore, if this bias can be quantified, it is recommended that methods be developed by which the impact of this bias can be reduced or eliminated to provide more accurate estimates of the population mean travel time.

## ACKNOWLEDGMENTS

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## APPENDIX. REFERENCES

- Boyce, D., Hicks, J., and Sen, A. K. (1991a). "In-vehicle navigation system requirements for monitoring link travel times in a dynamic route guidance system." *Proc., 70th Annu. Meeting of the Transp. Res. Board*.
- Boyce, D., Kirson, A., and Schofer, J. (1991b). "Design and implementation of ADVANCE: The Illinois dynamic navigation and route guidance program." *Proc., Vehicle Navigation and Information Sys. Conf., Vol. 1*, Society of Automotive Engineers, Warrendale, Pa., 415–426.
- Hurdle, V. F. (1984). "Signalized intersection delay models—A primer for the uninitiated." *Transp. Res. Rec. 971*, Transportation Research Board, Washington, D.C., 96–105.
- Sen, A., Soot, S., Ligas, J., and Tian, X. (1997a). "Arterial link travel time estimation: Probes, detectors and assignment-type models." *Proc., 70th Annu. Meeting of the Transp. Res. Board, Preprint No. 970943*.
- Sen, A., Thakuriah, P., Zhu, X., and Karr, A. (1997b). "Frequency of probe reports and variance of travel time estimates." *J. Transp. Engrg., ASCE*, 123(4), 290–297.
- Srinivasan, K. K., and Jovanis, P. P. (1996). "Oversaturation delay estimates with consideration of peaking." *Transp. Res. Rec. 1537*, Transportation Research Board, Washington, D.C., 15–22.
- Teply, S., Allingham, D. I., Richardson, D. B., and Stephenson, B. W. (1995). *Canadian capacity guide for signalized intersections*. Institute of Transportation Engineers, District 7, Canada.
- Turner, M. S., and Holdener, D. J. (1995). "Probe vehicle sample sizes for real-time information: The Houston experience." *Proc., Vehicle Navigation and Information Sys. (VNIS) Conf., IEEE*, New York, 3–9.
- Van Aerde, M., Hellinga, B., Baker, M., and Rakha, H. (1996). "INTEGRATION: Overview of simulation features." *Proc., 75th Annu. Meeting of the Transp. Res. Board*, Transportation Research Board, Washington, D.C.
- Van Aerde, M., Hellinga, B., Yu, L., and Rakha, H. (1993). "Vehicle probes as real-time ATMS sources of dynamic O-D and travel time data." *Large urban systems—Proc., ATMS Conf.*, 207–230.